Company

Address City, State Phone other

CS09 Ver 10.01.10

JOB TITLE Example 3 - 157' Building, flat terrain

JOB NO. SHEET NO. CALCULATED BY DATE DATE DATE

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# STRUCTURAL CALCULATIONS

FOR

# Example 3 - 157' Building, flat terrain

Guide to Wind Load Procedures of ASCE 7-02

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# **Code Search**

**Code:** ASCE 7 - 02

## Occupancy:

Occupancy Group = B Business

## **Occupancy Category & Importance Factors:**

Occupancy Category =	Π
Wind factor =	1.00
Snow factor =	1.00
Seismic factor =	1.00

## Type of Construction:

Fire Rating:

Roof =	0.0 hr
Floor =	0.0 hr

## **Building Geometry:**

Roof angle $(\theta)$	0.00 / 12	0.0 deg
Building length (L)	200.0 ft	
Least width (B)	100.0 ft	
Mean Roof Ht (h)	157.0 ft	
Parapet ht above grd	160.0 ft	
Minimum parapet ht	3.0 ft	

### Live Loads:

Roof	0 to 200 sf:	20 psf
	200 to 600 sf:	24 - 0.02Area, but not less than 12 psf
	over 600 sf:	12 psf

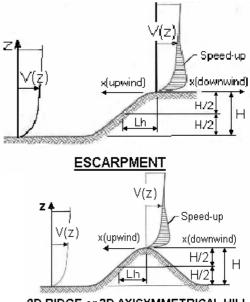
#### <u>Floor</u>

Typical Floor	50 psf
Lobbies & first floor corridors	100 psf
Corridors above first floor	80 psf
Mechanical	100 psf
Stairs & Exitways	100 psf
Balcony / Deck	50 psf
Partitions	20 psf

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# Wind Loads :

	1.00	
Importance Factor	1.00	
Basic Wind speed	120 mph	
Directionality (Kd)	0.85	
Exposure Category	В	
Enclosure Classif.	Partially Enclosed	
Internal pressure	+/-0.55	
Kh case 1	1.124	
Kh case 2	1.124	
Type of roof	Monoslope	
Topographic Factor (K	<u>zt)</u>	
Topography	Flat	
Hill Height (H)	80.0 ft	
Half Hill Length (Lh)	100.0 ft	
Actual H/Lh =	0.80	
Use H/Lh =	0.50	
Modified Lh =	160.0 ft	
From top of crest: x=	50.0 ft	
Bldg up/down wind?	downwind	
Drug up/ do mi milur	downinia	
H/Lh= 0.50	$K_1 =$	0.000
x/Lh = 0.31	$\mathbf{K}_2 =$	0.792
z/Lh = 0.98	$K_3 =$	1.000
At Mean Roof Ht:	<b>K</b> <sub>3</sub> –	1.000
110 1010001 110		1 00
Kzt =	$(1+K_1K_2K_3)^2 =$	1.00



2D RIDGE or 3D AXISYMMETRICA	<b>AL HILL</b>

Gust Effe	ct Factor
h =	157.0 ft
$\mathbf{B} =$	100.0 ft
/z (0.6h) =	94.2 ft

<b>Rigid Structure</b>		
$ \epsilon =$	0.33	
1 =	320 ft	
$z_{min} =$	30 ft	
c =	0.30	
$g_Q, g_v =$	3.4	
$L_z =$	453.9 ft	
Q =	0.83	
$I_z =$	0.25	
G =	0.83	

$$\label{eq:result} \begin{split} \text{Flexible structure if natural frequency} < 1 \text{ Hz (T} > 1 \text{ second}). \\ \text{However, if building h/B} < 4 \text{ then probably rigid structure (rule of thumb).} \\ \text{h/B} = 1.57 \\ \end{split}$$

G =	0.83	Using rigid structure formula
<b>u</b> –	0.05	Using rigid structure formula

Flexible or 1	Dynamically S	ensitive St	tructure		
Natural Frequency $(n_1) =$	0.0 Hz				
Damping ratio ( $\beta$ ) =	0				
/b =	0.45				
$/\alpha =$	0.25				
Vz =	102.9				
$N_1 =$	0.00				
$R_n =$	0.000				
$R_h =$	28.282	η =	0.000	h =	157.0 ft
$R_B =$	28.282	η =	0.000		
$R_L =$	28.282	η =	0.000		
$g_R =$	0.000				
R =	0.000				
G =	0.000				

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### **Enclosure Classification**

Test for Enclosed Building: A building that does not qualify as open or partially enclosed.

Test for Open Building: All walls are at least 80% open.

Ao  $\geq 0.8$ Ag

#### Test for Partially Enclosed Building:

	Input		Test	
Ao	0.0 sf	Ao ≥ 1.1Aoi	YES	
Ag	0.0 sf	Ao > 4' / 0.01Ag	NO	
Aoi	0.0 sf	Aoi / Agi $\leq 0.20$	NO	Building is NOT Partially Enclosed.
Agi	0.0 sf	-		-

Conditions to qualify as Partially Enclosed Building. Must satisfy all of the following:

Ao >= 1.1Aoi

Ao > smaller of 4' or 0.01 Ag

Aoi / Agi <= 0.20

Where:

Ao = the total area of openings in a wall that receives positive external pressure.

Ag = the gross area of that wall in which Ao is identified.

Aoi = the sum of the areas of openings in the building envelope (walls and roof) not including Ao.

Agi = the sum of the gross surface areas of the building envelope (walls and roof) not including Ag.

#### Reduction Factor for large volume partially enclosed buildings (Ri) :

If the partially enclosed building contains a single room that is unpartitioned, the internal pressure coefficient may be multiplied by the reduction factor Ri.

Total area of all wall & roof openings (Aog):		0 sf
Unpartitioned internal volume (Vi) :		0 cf
	Ri =	1.00

#### Altitude adjustment to constant 0.00256 :

Altitude =	0 feet	Average Air Density =	0.0765 lbm/ft <sup>3</sup>
Constant =	0.00256		

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#### Wind Loads - MWFRS all h (Enclosed/partially enclosed only)

Kh (case $2$ ) =	1.12	h =	157.0 ft	GCpi =	+/-0.55	
Base pressure $(q_h) =$	35.2 psf	ridge ht =	157.0 ft	G =	0.83	
Roof Angle =	0.0 deg	L =	200.0 ft	z for qi :	157.0 ft	use 90.00
Roof tributary area - (h/2)*L:	15700 sf	$\mathbf{B} =$	100.0 ft	qi =	30.0 psf f	for positive internal pressures
(h/2)*B:	7850 sf					

Surface Pressures (psf)	Win	d Normal to	o Ridge (psf)			Wind P	arallel to R	tidge (psf)	
	B/L =	0.50	h/L =	1.57		L/B =	2.00	h/L =	0.79
Surface	Ср	$q_h GC_p$	w/+q_iGC_{pi}	w/-qhGCpi	Dist.*	Ср	$q_h GC_p$	w/ + $q_i GC_{pi}$	w/ -q_hGC_{pi}
Windward Wall (WW)	0.80	23.5	see tabl	e below		0.80	23.5	see tab	le below
Leeward Wall (LW)	-0.50	-14.7	-31.2	4.7		-0.30	-8.8	-25.3	10.6
Side Wall (SW)	-0.70	-20.6	-37.1	-1.2		-0.70	-20.6	-37.1	-1.2
Leeward Roof (LR)		**				Inc	cluded in wi	ndward roof	
Windward Roof: 0 to h/2*	-1.04	-30.5	-47.1	-11.2	0 to h/2*	-0.98	-28.8	-45.3	-9.4
> h/2*	-0.70	-20.6	-37.1	-1.2	h/2 to h*	-0.79	-23.1	-39.6	-3.7
					h to 2h*	-0.61	-18.0	-34.6	1.3

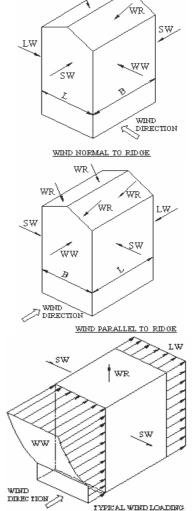
\*\*Roof angle < 10 degrees. Therefore, leeward roof is included in windward roof pressure zones.

\*Horizontal distance from windward edge

For monoslope roofs, entire roof surface is either windward or leeward surface.

LR

Windward Wall Pressures at "z" (psf)						Combined WW + LW		
			W	indward Wa	.11	Normal	Parallel	
Z	Kz	Kzt	$q_z GC_p$	$w/\!+\!q_iGC_{pi}$	w/-q_hGC_{pi}	to Ridge	to Ridge	
0 to 15'	0.57	1.00	12.0 psf	-4.5 psf	31.4 psf	26.7 psf	20.8 psf	
30.0 ft	0.70	1.00	14.6	-1.9	34.0	29.3	23.4	
50.0 ft	0.81	1.00	16.9	0.4	36.3	31.6	25.7	
80.0 ft	0.93	1.00	19.4	2.8	38.7	34.1	28.2	
120.0 ft	1.04	1.00	21.8	5.2	41.1	36.4	30.6	
157.0 ft	1.12	1.00	23.5	7.0	42.9	38.2	32.3	
	z 0 to 15' 30.0 ft 50.0 ft 80.0 ft 120.0 ft	z Kz   0 to 15' 0.57   30.0 ft 0.70   50.0 ft 0.81   80.0 ft 0.93   120.0 ft 1.04	z Kz Kzt   0 to 15' 0.57 1.00   30.0 ft 0.70 1.00   50.0 ft 0.81 1.00   80.0 ft 0.93 1.00   120.0 ft 1.04 1.00	z Kz Kzt $q_z GC_p$ 0 to 15' 0.57 1.00 12.0 psf   30.0 ft 0.70 1.00 14.6   50.0 ft 0.81 1.00 16.9   80.0 ft 0.93 1.00 19.4   120.0 ft 1.04 1.00 21.8	$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$



TYPICAL WIND LOADING

NOTE:

See figure 6-9 of ASCE7 for the application of full and partial loading of the above wind pressures. There are 4 different loading cases.

Parapet

Z	Kz	Kzt	qp (psf)
160.0 ft	1.13	1.00	35.4
	ard parapet: ard parapet:	63.7 psf -39.0 psf	× 1 /
Leewa	nu parapet.	-39.0 psi	(0Cpii - 1.1)

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### Wind Loads - Components & Cladding : h > 60'

Kh (case 1) = $($	1.12	h =	157.0 ft		
Base pressure (qh) =	35.2 psf	a =	10.0 ft		
Minimum parapet ht =	3.0 ft	GCpi =	+/-0.55		
Roof Angle =	0.0 deg	qi =	30.0 psf for		
		positive int	positive internal pressures		

Roof	GCp		Surfa	Surface Pressure (psf)			User input	
Area	10 sf	100 sf	500 sf	10 sf	100 sf	500 sf	50 sf	250 sf
Negative Zone 1	-1.40	-1.11	-0.90	-65.8 psf	-55.5 psf	-48.2 psf	-58.6 psf	-51.3 psf
Negative Zone 2	-2.30	-1.89	-1.60	-97.5 psf	-83.0 psf	-72.9 psf	-87.4 psf	-77.3 psf
Negative Zone 3	-2.30	-1.89	-1.60	-97.5 psf	-83.0 psf	-72.9 psf	-87.4 psf	-77.3 psf
Positive Zones 1-3	-	-	-	10.0 psf	10.0 psf	10.0 psf	10.0 psf	10.0 psf

157.0 ft

h =

Negative zone 3 = zone 2, since parapet >= 3ft.

#### **Parapet**

qp = 35.4  psf		Surface Pressure (psf)			
Solid Parapet Pressure	10 sf	100 sf	500 sf	40 sf	
CASE A : Interior zone:	113.3 psf	93.4 psf	77.9 psf	102.3 psf	
Corner zone:	113.3 psf	93.4 psf	77.9 psf	102.3 psf	
CASE B : Interior zone:	-63.7 psf	-54.9 psf	-46.0 psf	-59.9 psf	
Corner zone:	-95.6 psf	-76.1 psf	-56.7 psf	-87.2 psf	
	CASE A : Interior zone: Corner zone: CASE B : Interior zone:	Solid Parapet Pressure10 sfCASE A : Interior zone:113.3 psfCorner zone:113.3 psfCASE B : Interior zone:-63.7 psf	Solid Parapet Pressure10 sf100 sfCASE A : Interior zone:113.3 psf93.4 psfCorner zone:113.3 psf93.4 psfCASE B : Interior zone:-63.7 psf-54.9 psf	Solid Parapet Pressure 10 sf 100 sf 500 sf   CASE A : Interior zone: 113.3 psf 93.4 psf 77.9 psf   Corner zone: 113.3 psf 93.4 psf 77.9 psf   CASE B : Interior zone: -63.7 psf -54.9 psf -46.0 psf	

Walls	GCp			Surface Pressure at "h"			User input	
Area	20 sf	100 sf	500 sf	20 sf	100 sf	500 sf	28 sf	55 sf
Negative Zone 4	-0.90	-0.80	-0.70	-48.2 psf	-44.7 psf	-41.2 psf	-47.5 psf	-46.0 psf
Negative Zone 5	-1.80	-1.40	-1.00	-79.9 psf	-65.8 psf	-51.8 psf	-77.1 psf	-71.1 psf
Positive Zone 4 & 5	0.90	0.75	0.60	51.1 psf	45.8 psf	40.5 psf	50.0 psf	47.8 psf
110 mm 11		-					-	

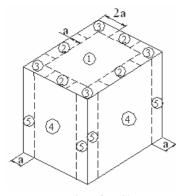
NOTE: Negative zones 4 & 5 pressures apply to all heights. Positive pressures vary with height, see below.

Wall surface pressure at 'z'		Positive zone 4 & 5 (psf)			User input			
Z	Kz	Kzt	qz (psf)	20	100	500	28 sf	55 sf
0 to 15'	0.70	1.00	22.0	39.1	35.8	32.5	38.5	37.1
30.0 ft	0.70	1.00	22.0	39.1	35.8	32.5	38.5	37.1
50.0 ft	0.81	1.00	25.4	42.2	38.4	34.6	41.5	39.8
80.0 ft	0.93	1.00	29.1	45.5	41.2	36.8	44.7	42.8
120.0 ft	1.04	1.00	32.6	48.7	43.8	38.9	47.8	45.7
157.0 ft	1.12	1.00	35.2	51.1	45.8	40.5	50.0	47.8

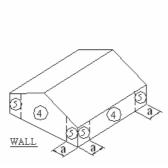
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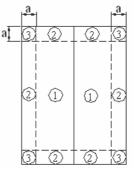
### **Location of Wind Pressure Zones**



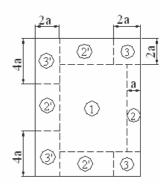
Roofs w/  $\theta \le 10^{\circ}$ and all walls **h** > 60'

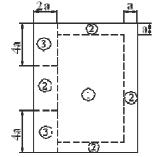


Walls h ≤ 60' & alt design h<90'

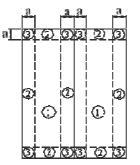


Gable, Sawtooth andMultispan Gable  $\theta \leq 7$  degrees &Monoslope  $\leq 3$  degrees $h \leq 60'$  & alt design h < 90'

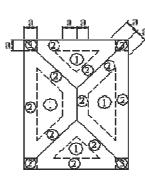




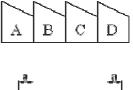
 $\label{eq:monoslope} \begin{array}{l} Monoslope \mbox{ roofs} \\ 10^\circ < \theta \leq 30^\circ \\ \mbox{ h \leq 60' \& alt design h<90' } \end{array}$ 

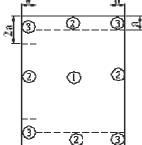


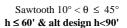
Multispan Gable & Gable  $7^{\circ} < \theta \le 45^{\circ}$ 

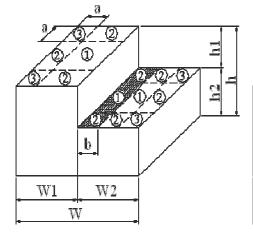


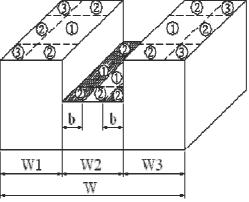
Hip  $7^\circ < \theta \le 27^\circ$ 











Stepped roofs  $\theta \le 3^{\circ}$ h  $\le 60'$  & alt design h<90'

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## **Roof Design Loads**

Items	Description Mult	ple	psf (max)	psf (min)
Roofing	3 ply felt & gravel		5.5	5.0
Decking	Metal Roof deck, 1.5, 22 ga.		1.7	1.2
Framing	Steel roof joists & girders		3.0	2.0
Insulation	Rigid insulation, per 1" x 2	.0	3.0	1.5
Ceiling	Suspended acoustical tile		1.8	1.0
Sprinklers	Sprinklers		2.0	0.0
Mech & Elec	Mech. & Elec.		2.0	0.0
			0.0	0.0
	Actual Dead I	Load	O 19.0	O <sub>10.7</sub>
	Use this DL ins	tead	<b>2</b> 0.0	<b>9</b> .0
	Live	e Load	20.0	0.0
	Snow	/ Load	0.0	0.0
	Wind (zone 2 -	100sf)	10.0	-83.0
ASD Loading	Dead + Live	e Load	40.0	-
	Dead + 0.75(Wind + Live	) Load	42.5	-
	0.6*Dead + Wind	l Load	-	-77.6
LRFD Loading	1.2D + 1.6 Lr +	0.8W	64.0	-
	1.2D + 1.6W +	0.5Lr	50.0	-
	0.9D +	1.6W	-	-124.7

#### **Roof Live Load Reduction**

<u>Roof angle</u> 0.00 / 12

0.0 deg

0 to 200 sf: 20.0 psf 200 to 600 sf: 24 - 0.02Area, but not less than 12 psf over 600 sf: 12.0 psf

	300 sf	18.00 psf
	400 sf	16.00 psf
	500 sf	14.00 psf
User Input:	450 psf	15.00 psf

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ltems	Description	Multiple	psf (max)	psf (min)
Flooring	Carpet & pad		1.0	1.0
Topping	Concrete regular per 1"	x 4.3	53.1	51.0
Decking	Metal Floor deck - 2", 20ga		2.0	1.5
Framing	Steel floor bms/joists & girders		8.0	5.0
Topping	Deflection Concrete	x 0.8	9.4	1.5
Ceiling	Suspended acoustical tile		1.8	1.0
Sprinklers	Sprinklers		2.0	0.0
Mech & Elec	Mech. & Elec.		2.0	0.0
Misc.	Misc.		0.5	0.0
			0.0	0.0
		Actual Dead Load	<b>9</b> 79.8	• 61.0
		Use this DL instead	O 100.0	O 50.0
		Partitions	20.0	0.0
		Live Load	50.0	0.0
		Total Live Load	70.0	0.0
		Total Load	149.8	61.0

IBC alternate procedure

## Floor Design Loads

#### FLOOR LIVE LOAD REDUCTION (not including partitions)

Company

NOTE: Not allowed for assembly occupancy or LL>100psf or passenger car garages, except may reduce columns 20% if 2 or more floors & non-assembly

	oro ce non assennory				
		Smallest of:			
$Lo(0.25+15/\sqrt{K_{LL}A_T})$		R= .08%(SF - 150)			
50 psf		R= 23.1(1+D/L) =	60.0%		
		R=40% beams; 60% columns			
2					
300 sf		R =	12.0%		
43.1 psf		Reduced live load: L =	44.0 psf		
4					
500 sf		R =	28.0%		
29.3 psf		Reduced live load: L =	36.0 psf		
	ELo(0.25+15/\\K <sub>LL</sub> A <sub>T</sub> ) 50 psf 2 300 sf 43.1 psf 4 500 sf	50 psf 2 300 sf 43.1 psf 4 500 sf	$ \begin{array}{c} \text{Smallest of:} \\ \text{Smallest of:} \\ \text{R} = .08\%(\text{SF} - 150) \\ \text{R} = 23.1(1 + D/L) = \\ \text{R} = 40\% \text{ beams; } 60\% \text{ columns} \\ \\ 300 \text{ sf} \\ \text{4} \\ 500 \text{ sf} \\ \text{R} = \\ \end{array} $		

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# CODE SUMMARY

Code:		ASCE 7 - 02	
Live Loads:			
Roof	0 to 200 sf: 200 to 600 sf: over 600 sf:	24 - 0.02Ar	ea, but not less than 12 psf
Typical Floor Lobbies & first floor of Corridors above first f		50 psf 100 psf 80 psf	
Mechanical Stairs & Exitways Balcony / Deck Partitions		100 psf 100 psf 50 psf 20 psf	
Dead Loads:			
Floor Roof		79.8 psf 20.0 psf	
Wind Design Data:			
Basic Wind speed Mean Roof Ht (h) Occupancy Category Importance Factor Exposure Category Enclosure Classif. Internal pressure Coef. Directionality (Kd)		120 mph 157.0 ft 1.00 B Partially Enc +/-0.55 0.85	losed
Roof Snow Loads:			
Design Roof Snow load Flat Roof Snow Load Ground Snow Load Rain on Snow Surcharge Importance Factor Snow Exposure Factor Thermal Factor Sloped-roof Factor		= Pf = Pg = I = Ct = Cs =	33.6 psf 40.0 psf 0.0 psf 1.00 1.00 1.20
Earthquake Design I	Data:		
Seismic Use Group : Importance Factor Mapped spectral respons	e accelerations	= I = Ss = S1 =	1.00 60.00 %g
Site Class Spectral Response Coef.		= Sds = Sd1 =	0.528
Seismic Design Category Basic Structural System Seismic Resisting Systen Design Base Shear Seismic Response Coef. Response Modification F	1	=	D Bearing Wall Systems Special reinforced concrete shear walls 0.036W 0.036
Analysis Procedure		=	Equivalent Lateral-Force Analysis

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#### CODE SUMMARY- continued

Component and cladding wind pressures

Roof		Surface Pressure (psf)		
	Area	10 sf	100 sf	500 sf
	Negative Zone 1	-65.8	-55.5	-48.2
	Negative Zone 2	-97.5	-83.0	-72.9
	Negative Zone 3	-97.5	-83.0	-72.9
	Positive Zones 1-3	10.0	10.0	10.0

Parapet	Solid Parapet Pressure (psf)		
Area	10 sf	100 sf	500 sf
CASE A: Interior zone	113.3	93.4	77.9
Corner zone	113.3	93.4	77.9
CASE B: Interior zone	-63.7	-54.9	-46.0
Corner zone	-95.6	-76.1	-56.7

Wall	Surface Pressure (psf)		
Area	20 sf	100 sf	500 sf
Negative Zone 4	-48.2	-44.7	-41.2
Negative Zone 5	-79.9	-65.8	-51.8
Positive Zone 4 & 5			
0 to 15	39.1	35.8	32.5
30 ft	39.1	35.8	32.5
50 ft	42.2	38.4	34.6
80 ft	45.5	41.2	36.8
120 ft	48.7	43.8	38.9
h = 157 ft	51.1	45.8	40.5