JOB TITLE Example 6&7

Company

Address City, State Phone other

CS09 Ver 10.01.10

JOB NO. SHEET NO. CALCULATED BY DATE DATE DATE

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STRUCTURAL CALCULATIONS

FOR

Example 6&7 20' eave height using MWFRS all heights procedure 20' eave height using MWFRS < 60' procedure

Guide to Wind Load Procedures of ASCE 7-02

Company Address City, State Phone other

JOB TITLE Example 6&7

 JOB NO.
 SHEET NO.

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Code Search

Code: ASCE 7 - 02

Occupancy:

Occupancy Group = B Business

Occupancy Category & Importance Factors:

Occupancy Category =	Π
Wind factor =	1.00
Snow factor =	1.00
Seismic factor =	1.00

Type of Construction:

Fire Rating:

Roof =	0.0 hr
Floor =	0.0 hr

Building Geometry:

Roof angle (θ)	4.00 / 12	18.4 deg
Building length (L)	250.0 ft	
Least width (B)	200.0 ft	
Mean Roof Ht (h)	36.7 ft	
Parapet ht above grd	0.0 ft	
Minimum parapet ht	0.0 ft	

Live Loads:

Roof	0 to 200 sf:	20 psf
	200 to 600 sf:	24 - 0.02Area, but not less than 12 psf
	over 600 sf:	12 psf

<u>Floor</u>

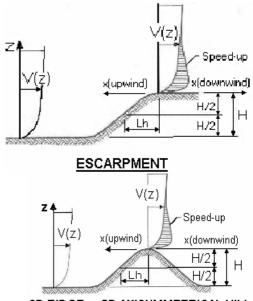
Typical Floor	125 psf
Walkways & elevated platforms (non e	60 psf

Mechanical	125 psf
Stairs & Exitways	100 psf
Balcony / Deck	
Partitions	N/A

Company	JOB TITLE Example	
Address		
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Wind Loads :

	1.00	
Importance Factor	1.00	
Basic Wind speed	90 mph	
Directionality (Kd)	0.85	
Exposure Category	С	
Enclosure Classif.	Enclosed Building	
Internal pressure	+/-0.18	
Kh case 1	1.025	
Kh case 2	1.025	
Type of roof	Gable	
Topographic Factor (K	<u>zt)</u>	
Topography	Flat	
Hill Height (H)	80.0 ft	
Half Hill Length (Lh)	100.0 ft	
Actual H/Lh =	0.80	
Use H/Lh =	0.50	
Modified Lh =	160.0 ft	
From top of crest: x=	50.0 ft	
Bldg up/down wind?	downwind	
H/Lh = 0.50	$K_1 =$	0.000
	1	
x/Lh = 0.31	$K_2 =$	0.792
z/Lh = 0.23	$K_3 =$	1.000
At Mean Roof Ht:		
Kzt =	$(1+K_1K_2K_3)^2 =$	1.00



2D RIDGE or 3D AXISYMMETRICAL H	ILL

Gust	Effect	Factor
ł	1 =	36.7 ft
E	8 =	200.0 ft
/z (0.6h)) =	22.0 ft

Rigid Structure	
$ \epsilon =$	0.20
1 =	500 ft
$z_{min} =$	15 ft
c =	0.20
$g_Q, g_v =$	3.4
$L_z =$	461.1 ft
Q =	0.84
$I_z =$	0.21
G =	0.84

Flexible structure if natural frequent	ncy < 1 Hz (T > 1 second).
However, if building $h/B < 4$ then	probably rigid structure (rule of thumb).
h/B = 0.18	Therefore, probably rigid structure

G =	0.85	Using rigid structure default
-		0 0

Flexible or Dynamically Sensitive Structure									
Natural Frequency $(n_1) =$	0.0 Hz								
Damping ratio (β) =	0								
/b =	0.65								
$/\alpha =$	0.15								
Vz =	80.6								
$N_1 =$	0.00								
$R_n =$	0.000								
$R_h =$	28.282	η =	0.000	h =	36.7 ft				
$R_B =$	28.282	η =	0.000						
$R_L =$	28.282	η =	0.000						
$g_R =$	0.000								
R =	0.000								
G =	0.000								

Company	JOB TITLE Example 6	&7
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Enclosure Classification

Test for Enclosed Building: A building that does not qualify as open or partially enclosed.

Test for Open Building: All walls are at least 80% open.

Ao ≥ 0.8 Ag

Test for Partially Enclosed Building:

	Input		Test	
Ao	0.0 sf	Ao ≥ 1.1Aoi	YES	
Ag	0.0 sf	Ao > 4' / 0.01Ag	NO	
Aoi	0.0 sf	Aoi / Agi ≤ 0.20	NO	Building is NOT Partially Enclosed.
Agi	0.0 sf	-		-

Conditions to qualify as Partially Enclosed Building. Must satisfy all of the following:

Ao >= 1.1Aoi

Ao > smaller of 4' or 0.01 Ag

Aoi / Agi <= 0.20

Where:

Ao = the total area of openings in a wall that receives positive external pressure.

Ag = the gross area of that wall in which Ao is identified.

Aoi = the sum of the areas of openings in the building envelope (walls and roof) not including Ao.

Agi = the sum of the gross surface areas of the building envelope (walls and roof) not including Ag.

Reduction Factor for large volume partially enclosed buildings (Ri) :

If the partially enclosed building contains a single room that is unpartitioned, the internal pressure coefficient may be multiplied by the reduction factor Ri.

Total area of all wall & roof openings (Aog):		0 sf
Unpartitioned internal volume (Vi):		0 cf
	Ri =	1.00

Altitude adjustment to constant 0.00256 :

Altitude =	0 feet	Average Air Density =	0.0765 lbm/ft ³
Constant =	0.00256		

Company	JOB TITLE Example 6&	27
Address		
City, State	JOB NO.	SHEET NO.
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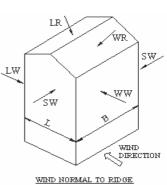
Wind Loads - MWFRS all h (Enclosed/partially enclosed only)

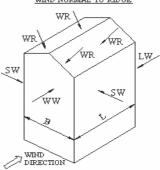
Kh (case 2) =	1.02	h =	36.7 ft	GCpi =	+/-0.18
Base pressure $(q_h) =$	18.1 psf	ridge ht =	53.4 ft	G =	0.85
Roof Angle =	18.4 deg	L =	250.0 ft	qi = qh	
Roof tributary area - (h/2)*L:	4588 sf	B =	200.0 ft		
(h/2)*B:	3670 sf				

Surface Pressures (psf)	Wind Normal to Ridge (psf)				Wind P	arallel to R	tidge (psf)		
	B/L =	0.80	h/L =	0.18		L/B =	1.25	h/L =	0.15
Surface	Ср	$q_h GC_p$	$w/+q_iGC_{pi}$	w/-qhGCpi	Dist.*	Ср	$q_h GC_p$	w/ + $q_i GC_{pi}$	w/ -q_hGC_{pi}
Windward Wall (WW)	0.80	12.3	see tabl	e below		0.80	12.3	see tabl	e below
Leeward Wall (LW)	-0.50	-7.7	-10.9	-4.4		-0.45	-6.9	-10.2	-3.7
Side Wall (SW)	-0.70	-10.7	-14.0	-7.5		-0.70	-10.7	-14.0	-7.5
Leeward Roof (LR)	-0.57	-8.7	-12.0	-5.5		Inc	cluded in wi	ndward roof	
Windward Roof neg press.	-0.36	-5.6	-8.8	-2.3	0 to h/2*	-0.90	-13.8	-17.1	-10.6
Windward Roof pos press.	0.14	2.1	-1.1	5.4	h/2 to h*	-0.90	-13.8	-17.1	-10.6
					h to 2h*	-0.50	-7.7	-10.9	-4.4
					$> 2h^*$	-0.30	-4.6	-7.9	-1.4

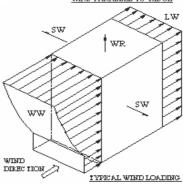
*Horizontal distance from windward edge

	Windward Wall Pressures at "z" (psf)							Combined WW + LW	
				W	indward Wa	.11	Normal	Parallel	
	Z	Kz	Kzt	$q_z GC_p$	$w/\!+\!q_iGC_{pi}$	w/-q_hGC_{pi}	to Ridge	to Ridge	
-	0 to 15'	0.85	1.00	10.2 psf	6.9 psf	13.4 psf	17.9 psf	17.1 psf	
	20.0 ft	0.90	1.00	10.8	7.6	14.1	18.5	17.7	
	25.0 ft	0.95	1.00	11.3	8.1	14.6	19.0	18.2	
	30.0 ft	0.98	1.00	11.8	8.5	15.0	19.4	18.7	
h=	36.7 ft	1.02	1.00	12.3	9.0	15.5	20.0	19.2	
ridge =	53.4 ft	1.11	1.00	13.3	10.0	16.5	21.0	20.2	





WIND PARALLEL TO RIDGE



NOTE:

See figure 6-9 of ASCE7 for the application of full and partial loading of the above wind pressures. There are 4 different loading cases.

Parapet

Z	Kz	Kzt	qp (psf)	
0.0 ft	0.85	1.00	0.0	-
	ard parapet: ard parapet:	0.0 psf 0.0 psf	× 1	

Company	JOB TITLE Example 6&7			
Address				
City, State	JOB NO.	SHEET NO.		
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Wind Loads - MWFRS h <60' (Low-rise Buildings) Enclosed/partially enclosed only

			<u>6</u>	W HISC D	unungs		Scu/pur	liany cher		<u>iiy</u>
	4	8	7×1	-ZONE 2: les			20			Torsional loads are
		$X \swarrow$		0.5 Bor 2.5) wit 2 is negati		~	-1,			25% of zones 1 - 4.
Æ	3E	11	$Z^* >$	∑°	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	<u></u>	$\sum \gamma$	6		See code for loading
	<u>Alianaj</u>	2 /2E			- VE 🔊		<u>~</u> *			diagram
	. e>						E V			
_	> ?	×	\sim							
5	\sim	◟▨ּּ<			5	শ্ৰ	1	>		
		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	WIND I	urec ticat					CORPC TICON	
PER IO:	PERIOCULA.R RIDGE				Paralli Ridge	ILIO				
-	Transverse	Direction			<u> 1</u>	Longitudi	nal Directio	<u>on</u>		
	Kz = Kh	(case 1) =	1.02					Edge Strip	(a)	14.7 ft
	Base press		18.1 psf					End Zone	(2a)	29.4 ft
		GCpi =	+/-0.18					Zone 2 length		91.8 ft
Г			erse Direct	ion	Longi	tudinal D	irection	_		
	-		rpendicular $\theta =$			$\theta = 0.0$				
	Surface	GCpf	w/-GCpi	w/+GCpi	GCpf	w/-GCpi	w/+GCpi			
	1	0.52	0.70	0.34	0.40	0.58	0.22			
	2	-0.69	-0.51	-0.87	-0.69	-0.51	-0.87			
	3	-0.47	-0.29	-0.65	-0.37	-0.19	-0.55			
	4	-0.42	-0.24	-0.60	-0.29	-0.11	-0.47			
	5	-0.45	-0.27	-0.63	-0.45	-0.27	-0.63			
_	6	-0.45	-0.27	-0.63	-0.45	-0.27	-0.63			
	1E	0.78	0.96	0.60	0.61	0.79	0.43			
	2E	-1.07	-0.89	-1.25	-1.07	-0.89	-1.25			
	3E	-0.67	-0.49	-0.85	-0.53	-0.35	-0.71			
	4E	-0.62	-0.44	-0.80	-0.43	-0.25	-0.61			
L	,	Wind Surfa	ice pressure	es (psf)						
Г	1		12.6	6.1		10.5	4.0			
	2		-9.2	-15.7		-9.2	-15.7			
	3		-5.2	-11.7		-3.4	-9.9			
	4		-4.3	-10.8		-2.0	-8.5			
	5		-4.9	-11.4		-4.9	-11.4			
_	6		-4.9	-11.4		-4.9	-11.4			
	1E		17.3	10.8		14.3	7.8			
	2E		-16.1	-22.6		-16.1	-22.6			
	3E		-8.9	-15.4		-6.3	-12.8			
	4E		-7.9	-14.4		-4.5	-11.0			
							WINDWAF			
Wind	lward roof o	overhangs:	12.3	psf (upwar	rd) add to wi	ndward	OVERHAN		ARD ROOF	LEEWARD ROOF
	_			roof p	ressure			1 + 1 + + + +	VER	FICAL
	Parapet						tion to the second	Ţ		
		d parapet:			n = +1.8)			LNOZ	_	
	Leeward parapet: $0.0 \text{ psf}  (\text{GCpn} = -1.1)$					MALL	B/2 or Z	ONE 2		
Horizon	Horizontal MWFRS Simple Diaphragm Pressures (psf)							- + <del>,,,,,,</del> ,		
-	Transverse direction (normal to L)						TRAI	NSVERSE	ELEVATION	
	Interior Zone: Wall 16.8 psf						WINDW	ARD ROOF	LEEWARD ROOF	
	Roof -4.0 psf						$\downarrow\downarrow\downarrow\downarrow\downarrow\downarrow\downarrow\downarrow\downarrow\downarrow\downarrow$	VERI	* * * ! * * * * *	
	End Zone: Wall 25.3 psf					[ <b></b> ],		VERI		
		Roof	-7.2	-					ONE 2	
	Longitudin		n (parallel to	-			MALL			
	Interior Zo		1 ( <b>parallel u</b> 12.5							
		one: Wall	12.3							<u>AL ELEVATION</u>
	Enu Zu	nic. wali	10.0	Par						

Company	
Address	

JOB TITLE Example 6&7

JOB NO.	SHEET NO.
CALCULATED BY	DATE
CHECKED BY	DATE
	CALCULATED BY

## Wind Loads - Components & Cladding : h <= 60'

Kh (case 1) $=$	1.02	$\mathbf{h} =$	36.7 ft
Base pressure (qh) =	18.1 psf	a =	14.7 ft
Minimum parapet ht =	0.0 ft	GCpi =	+/-0.18
Roof Angle =	18.4 deg		
Type of $roof = C$	able		

GCp +/- GCpi			Surfa	ce Pressure (	User input		
10 sf	50 sf	100 sf	10 sf	50 sf	100 sf	20 sf	208 sf
-1.08	-1.01	-0.98	-19.5 psf	-18.2 psf	-17.7 psf	-19.0 psf	-17.7 psf
-1.88	-1.53	-1.38	-34.0 psf	-27.6 psf	-24.9 psf	-31.2 psf	-24.9 psf
-2.78	-2.36	-2.18	-50.2 psf	-42.6 psf	-39.4 psf	-47.0 psf	-39.4 psf
0.68	0.54	0.48	12.3 psf	10.0 psf	10.0 psf	11.2 psf	10.0 psf
-2.20	-2.20	-2.20	-39.7 psf	-39.7 psf	-39.7 psf	-39.7 psf	-39.7 psf
-3.70	-2.86	-2.50	-66.8 psf	-51.7 psf	-45.2 psf	-60.3 psf	-45.2 psf
	10 sf -1.08 -1.88 -2.78 0.68 -2.20	10 sf         50 sf           -1.08         -1.01           -1.88         -1.53           -2.78         -2.36           0.68         0.54           -2.20         -2.20	10 sf         50 sf         100 sf           -1.08         -1.01         -0.98           -1.88         -1.53         -1.38           -2.78         -2.36         -2.18           0.68         0.54         0.48           -2.20         -2.20         -2.20	10 sf         50 sf         100 sf         10 sf           -1.08         -1.01         -0.98         -19.5 psf           -1.88         -1.53         -1.38         -34.0 psf           -2.78         -2.36         -2.18         -50.2 psf           0.68         0.54         0.48         12.3 psf           -2.20         -2.20         -2.20         -39.7 psf	10 sf         50 sf         100 sf         10 sf         50 sf           -1.08         -1.01         -0.98         -19.5 psf         -18.2 psf           -1.88         -1.53         -1.38         -34.0 psf         -27.6 psf           -2.78         -2.36         -2.18         -50.2 psf         -42.6 psf           0.68         0.54         0.48         12.3 psf         10.0 psf           -2.20         -2.20         -2.20         -39.7 psf         -39.7 psf	10 sf         50 sf         100 sf         10 sf         50 sf         100 sf           -1.08         -1.01         -0.98         -19.5 psf         -18.2 psf         -17.7 psf           -1.88         -1.53         -1.38         -34.0 psf         -27.6 psf         -24.9 psf           -2.78         -2.36         -2.18         -50.2 psf         -42.6 psf         -39.4 psf           0.68         0.54         0.48         12.3 psf         10.0 psf         10.0 psf           -2.20         -2.20         -2.20         -39.7 psf         -39.7 psf         -39.7 psf	10 sf         50 sf         100 sf         10 sf         50 sf         100 sf         20 sf           -1.08         -1.01         -0.98         -19.5 psf         -18.2 psf         -17.7 psf         -19.0 psf           -1.88         -1.53         -1.38         -34.0 psf         -27.6 psf         -24.9 psf         -31.2 psf           -2.78         -2.36         -2.18         -50.2 psf         -42.6 psf         -39.4 psf         -47.0 psf           0.68         0.54         0.48         12.3 psf         10.0 psf         10.0 psf         11.2 psf           -2.20         -2.20         -2.20         -39.7 psf         -39.7 psf         -39.7 psf         -39.7 psf

Overhang pressures in the table above assume an internal pressure coefficient (Gcpi) of 0.0

#### <u>Parapet</u>

qp = 0.0 psf		Surfa	User input		
	Solid Parapet Pressure	10 sf	100 sf	500 sf	40 sf
CASE A = pressure towards building	CASE A : Interior zone:	0.0 psf	0.0 psf	0.0 psf	0.0 psf
CASE B = pressure away from building	Corner zone:	0.0 psf	0.0 psf	0.0 psf	0.0 psf
	CASE B : Interior zone:	0.0 psf	0.0 psf	0.0 psf	0.0 psf
	Corner zone:	0.0 psf	0.0 psf	0.0 psf	0.0 psf

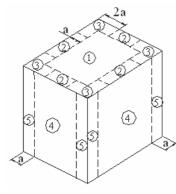
Walls		GCp +/- GCp	i	Surfa	ce Pressure a	User input		
Area	10 sf	100 sf	500 sf	10 sf	100 sf	500 sf	15 sf	208 sf
Negative Zone 4	-1.28	-1.10	-0.98	-23.1 psf	-19.9 psf	-17.7 psf	-22.6 psf	-18.9 psf
Negative Zone 5	-1.58	-1.23	-0.98	-28.5 psf	-22.2 psf	-17.7 psf	-27.4 psf	-20.1 psf
Positive Zone 4 & 5	1.18	1.00	0.88	21.3 psf	18.1 psf	15.9 psf	20.8 psf	17.1 psf

Company Address City, State Phone other

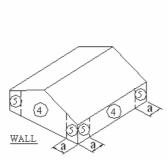
#### JOB TITLE Example 6&7

JOB NO.	SHEET NO.
CALCULATED BY	DATE
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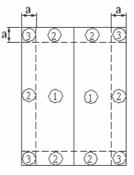
### **Location of Wind Pressure Zones**



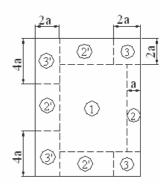
Roofs w/  $\theta \le 10^{\circ}$ and all walls **h** > 60'

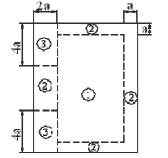


Walls h ≤ 60' & alt design h<90'

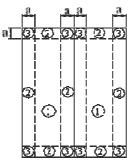


Gable, Sawtooth andMultispan Gable  $\theta \leq 7$  degrees &<br/>Monoslope  $\leq 3$  degrees<br/>h  $\leq 60'$  & alt design h<90'</td>

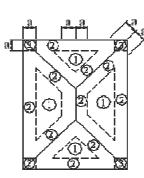




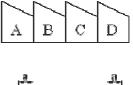
 $\label{eq:monoslope} \begin{array}{l} Monoslope \mbox{ roofs} \\ 10^\circ < \theta \leq 30^\circ \\ \mbox{ h \leq 60' \& alt design h<90' } \end{array}$ 

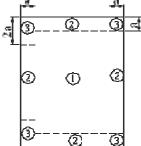


Multispan Gable & Gable  $7^{\circ} < \theta \le 45^{\circ}$ 

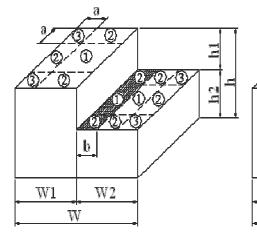


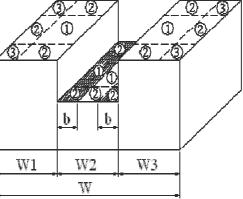
Hip 7° <  $\theta \leq 27^{\circ}$ 





Sawtooth  $10^{\circ} < \theta \le 45^{\circ}$ h  $\le 60'$  & alt design h<90'





Stepped roofs  $\theta \le 3^{\circ}$ h  $\le 60'$  & alt design h<90'